OPERATION & MAINTENANCE MANUAL

DFI No. D00084 Facility Type: Detention Pond



JUNE, 2011

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1. Identification

Drainage Facility ID (DFI):	DFI D00084
Facility Type:	Detention Pond
Construction Drawings:	(V-File Number) 37V-041
Location:	District: 2B (Old 2A)
	Highway No.: 047
	Mile Post: 68.36 / 68.36 (beg./end)
	Description: This facility is located at the southeastern quadrant of OR26 (Hwy 047) and SW Cedar Hills Blvd. Access may be obtained from access pads located at either

the south or north sides of the facility.

2. Facility Contact Information

Contact the Engineer of Record, Region Technical Center, or Geo-Environmental's Senior Hydraulics Engineer for:

- Operational clarification
- Maintenance clarification
- Repair or restoration assistance

Engineering Contacts:

Region Technical Center Hydro Unit Manager

Or

Geo-Environmental Senior Hydraulics Engineer (503) 986-3365.

3. Construction

Engineer of Record:	ODOT Designer – [Region 1 Tech. Center, Henry Minton Allen, 503-731-8200.
Facility construction:	March 2004
Contractor:	Mowatt Construction Company.

4. Storm Drain System and Facility Overview

A detention pond is designed to control the quantity of runoff, by reducing the peak discharge and only detaining runoff for some short period of time. These facilities are designed to store and gradually release or attenuate stormwater runoff via a control structure or release mechanism, and completely drain after the design storm has passed. The most common detention facilities include:

- Dry ponds these are depressed storage areas that store runoff during wet weather and are dry the rest of the time. Usually they are earthen depressions.
- Tanks these are underground storage facilities that are typically constructed from large diameter pipe.
- Vaults these are enclosed underground storage facilities. They are typically constructed from reinforced concrete.

This facility is located at the southeastern quadrant of OR26 (Hwy 047) and SW Cedar Hills Blvd. Access may be obtained from the south side of the facility from SW Butner Rd. – a frontage road adjacent to SW Cedar Hills Blvd. Additional access may be obtained from the north side of the facility from a maintenance pad located along the eastbound on-ramp to OR26 (Hwy 047).

Stormwater runoff from US26 (Hwy 047) is collected by a series of inlets along the south side of the highway and along the eastbound onramp to US26 (Hwy 047) for an approximate distance of 1,900 feet east of the facility. The drainage is conveyed by a 12-inch storm pipe where a portion of it is bypassed into a treatment swale (DFI D00083) at a high low flow splitter (See Point A on the Operational Plan). The water is then detained through the detention pond.

The detention pond discharges to an outlet control structure (combination of two ditch inlets) into the backside of a catch basin located on the north east corner of Cedar Hills Boulevard and SW Butner Road; see points E and F on the Operational Plan, Appendix A. This catch basin drains into a 24-inch storm pipe in Cedar Hills Blvd that drains towards the north.

The detention pond contains a low flow channel (riprap) that accommodates the flow prior to detention at the outlet structure. Refer to Photo 1 for a photograph of this channel.

A. Maintenance equipment access:

Access may be obtained from the south side of the facility from SW Butner Rd. – a frontage road adjacent to SW Cedar Hills Blvd. Additional access may be obtained from the north side of the facility from a maintenance pad located along the eastbound on-ramp to OR26 (Hwy 047).

- B. Heavy equipment access into facility:
 - ⊠ Allowed (no limitations)
 - □ Allowed (with limitations)
 - □ Not allowed
- C. Special Features:
 - □ Amended Soils
 - □ Porous Pavers
 - □ Liners
 - \Box Underdrains



Photo 1: Detention pond facility looking west towards Cedar Hills Blvd. The riprap in the bottom of the pond forms a low-flow channel.



Photo 2: Access maintenance pad on eastbound onramp to US 26 (Hwy 47) looking east.



Photo 3: Control Outlet Structure to the Detention Pond; Point E, Ops Plan.

5. Facility Haz Mat Spill Feature(s)

The detention pond can be used to store a volume of liquid by blocking the 12-inch diameter outlet pipe located at the outlet structure of the detention pond. This pipe and the outlet structure are noted as Point E on the Operational Plan, Appendix A. Another option may include blocking the grated inlets to the outlet structure through the use of either sandbags, or a metal plate.

6. Auxiliary Outlet (High Flow Bypass)

Auxiliary Outlets are provided if the primary outlet control structure can not safely pass the projected high flows. Broad-crested spillway weirs and over flow risers are the two most common auxiliary outlets used in

stormwater treatment facility design. The auxiliary outlet feature is either a part of the facility or an additional storm drain feature/structure.

The auxiliary outlet feature for this facility is:

 \boxtimes Designed into facility

A secondary auxiliary inlet/outlet grated catch basin has been designed as part of the facility's outlet control structure, and acts as an emergency overflow in the event the primary outlet control device is plugged.

Before flows ever reach the higher level of the secondary inlet/outlet device, however, they are typically released through a primary inlet/outlet grated catch basin located below the secondary device. If runoff should ever exceed the water quality event, where flows normally are directed to the lower primary outlet, the pond level will rise and flows will be released through the secondary auxiliary inlet/outlet device located just above the primary outlet.

 \Box Other, as noted below

7. Maintenance Requirements

Routine maintenance table for non-proprietary stormwater treatment and storage/detention facilities have been incorporated into ODOT's Maintenance Guide. These tables summarize the maintenance requirements for ponds, swales, filter strips, bioslopes, and detention tanks and vaults. Special maintenance requirements in addition to the routine requirements are noted below when applicable.

The ODOT Maintenance Guide can be viewed at the following website:

http://www.oregon.gov/ODOT/HWY/OOM/MGuide.shtml

Maintenance requirements for proprietary structures, such as underground water quality manholes and/or vaults with filter media are noted in Appendix C when applicable.

The following stormwater facility maintenance table (See ODOT Maintenance Guide) should be used to maintain the facility outlined in this Operation and Maintenance Manual or follow the Maintenance requirements outlined in Appendix C when proprietary structure is selected below:

- ⊠ Table 1 (general maintenance)
- \boxtimes Table 2 (stormwater ponds)
- □ Table 3 (water quality or biofiltration swales)

□ Table 4 (water quality filter strips)

 \Box Table 5 (water quality bioslopes)

 \Box Table 6 (detention tank)

□ Table 7 (detention vault)

- □ Appendix C (proprietary structure)
- □ Special Maintenance requirements:

Note: Special maintenance Requirements Require Concurrence from ODOT SR Hydraulics Engineer.

8. Waste Material Handling

Material removed from the facility is defined as waste by DEQ. Refer to the roadwaste section of the ODOT Maintenance Yard Environmental Management System (EMS) Policy and Procedures Manual for disposal options: <u>http://egov.oregon.gov/ODOT/HWY/OOM/EMS.shtml</u>

Contact any of the following for more detailed information about management of waste materials found on site:

ODOT Clean Water Unit	(503) 986-3008
ODOT Statewide Hazmat Coordinator	(503) 229-5129
ODOT Region Hazmat Coordinator	(503) 731-8290
ODEQ Northwest Region Office	(503) 229-5263

Appendix A

Content:

• Operational Plan and Profile Drawing(s)



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Appendix B

Content:

- ODOT Project Plan Sheets
 - Cover/Title Sheet
 - Water Quality/Detention Plan Sheets
 - Other Details

	INDEX OF SHEETS								
SHEET NO.	DESCRIPTION								
1	Title Sheet								
1A. 1A-2	Index Of Sheets Cont'd.								
1A-3 Std. Drg. Nos.									
1B	Sheet Layout								
2,2A,2A-2									
Thru	hru Typical Sections								
2A-65 Incl.									
2B, 2B-2									
Thru	Details								
28-18 Incl.									
20,20-2	Traffic Control Details								
2CA, 2CA-2,									
2CA-2A.	Traffic Control Plans - Nurray Work Area								
2CA-3 Thru	That is control thans mainly work Area								
2CA-57 Incl.									
2CB . 2CB-2									
Thru	Traffic Control Plans – Cornell Work Area								
2CB-12 Incl.									
2D.2D-2.									
Thru	Pipe Data Sheet								
2D-12.Incl.									

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US26: CORNELL RD. -**OR217 (BEAVERTON) SEC.** SUNSET HIGHWAY

MARCH 2004





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	Sta. "CBR" 95+770.98 To Sta. "CBR" 95+809.47, Lt. Const. Conc. Shldr. Barrier (Reflectorized) - 38.4 m Grout Barrier (Plug Scuppers) Flare Rate=0, W=0, E=0.6 m
	Remove Extg. Curb Const. Mountable Curb & Gutter (For Details, See Sht. 2B-3) (See Drg. No. RD700)
Mal	3 Sta. "CBR" 95+876.38 To Sta. "CBR" 95+956.00, Rt. Const. Cast-In-Place Conc. Shidr. Barrier (Reflectorized) - 84.4 m Flare Rate=20:1, W=0.61 m, E=0 Protect Leading End W/Extg. Impact Attenuator (See Drg. No. RD505)
AL DETAILS CHECKED	A Sta. "BR" 95+875.55 To Sta. "BR" 95+898.83, Lt. Const. Conc. Shidr. Barrier (Reflectorized) -23.3 m Pin Barrier To Rdwy. Grout Barrier (Plug Scuppers) Flare Rate=0.W=0.E=0.6 m Const. Conc. Barrier Transition To Bridge Rail
STRUCTUR	 Inst. Impact Attenuator Obstacle Width - 0.6 m Number Of Bays - 3 (For Details, See Sht. 2B-8)
	6 Bridge No. 19808 Sta. "LW" 95+880.00 Const. Sign Truss And Footings (For Drg. Nos., See Sht. 1A-2)
	 Bridge No. 9345 Sta. "LW" 95+907.73 To Sta. "LW" 95+967.47 Remove Extg. Bridge Rail Remove Extg. Asph. Conc. Surfacing Const. Bridge Rail Const. Structure Const. Structure Const. Structurel Overlay Const. Reinf. Panel At Bridge End W/Bridge Rail - 2 (For Drg. Nos., See Sht. 1A)
	8 Remove Extg. Curb Const.Low Profile Mountable Curb

(9) Const. Type CL-6 Fence

(10) See Sht. 16, Note 2 Const. Conc. Median Barrier

(I) See Sht. 17, Note 19 Const. Single Slope Conc. Barrier

12 See Sht. 17, Note 5 Const. Conc. Shldr. Barrier

(13) See Sht. 18E, Note 1 Const. Conc. Shidr. Barrier

(14) See Sht. 14, Note 5 Const. Soundwall No. 515

- (5) Sta. "CBR" 95+809.47 To Sta. "CBR" 95+857.71, Lt. Const. Cast-In-Place Conc. Shldr. Barrier (Reflectorized) - 46.5 m Flare Rate=20:1, W=2.3 m, E=0.6 m Const. Conc. Barrier Buried Terminal (For Details, See Sht. 2B-11) (See Drg. Nos. RD505 & RD510)
- (16) Remove Extg. Conc. Traffic Separator Const. Type "C" Traffic Separator (600 mm Width) With 300 mm Drain (See Drg. No. RD705)

(17) See Sht. 16, Note 9 Remove Extg. Conc. Barrier

 (18) Sta. "BR" 95+850.03, Rt. To Sta. "BR" 96+022.12, Lt. Remove Extg. Conc. Barrier - 169 m

 Sta. "LW" 96+012.92, Lt. To Sta. "LW" 96+197.56, Rt. Remove Extg. Conc. Barrier - 185 m

 Sta. "LE" 95+861.18 To Sta. "LE" 95+907.42, Rt. Remove Extg. Guardrail - 47 m

(21) Sta. "LE" 95+861.48 To Sta. "LE" 95+907.50, Lt. Remove Extg. Guardrail - 47 m Sta. "CBR" 95+896.40 To Sta. "CBR" 95+954.31. Rt. Remove Extg. Guardrail - 62 m

 (23) Sta. "LW" 95+967.85 To Sta. "LW" 95+013.73.Lt. Remove Extg. Guardrail - 47 m

 Sta. "SC" 95+984.57 To Sta. "SC" 96+161.83. Lt. Remove Extg. Guardrail - 180 m

25 Remove Extg. Curb

(26) Remove Extg. Conc. Traffic Separator

 Sta. "BR" 95+976.91 To Sta. "BR" 95+989.29. Lt. Const. Conc. Shidr. Barrier (Reflectorized) - 11.8 m Pin Barrier To Rdwy. Grout Barrier (Plug Scuppers) Flare Rate=0, W=0, E=0.6 m Const. Conc. Barrier Transition To Bridge Rail

 (28) Sta. "BR" 95+989.29 To Sta. "BR" 96+024.96, Lt. Const. Cast-In-Place Conc. Shldr. Barrier (Reflectorized) - 32.0 m Flare Rate=0.W=0, E=0.6 m

 Sta. "LE" 95+976.37 To Sta. "LE" 96+195.29, Rt. Const. Conc. Shldr. Barrier (Reflectorized) - 218.9 m Grout Barrier (Plug Scuppers) Flare Rate=0, W=0, E=0 Const. Conc. Barrier Transition To Bridge Rail

 Sta. "SC" 96+000.06 To Sta. "SC" 96+099.79, Lt. Const. Conc. Shldr. Barrier (Reflectorized) - 99.7 m Pin Barrier To Rdwy. Grout Barrier (Plug Scuppers) Flare Rate=0, W=0, E=0.6 m

37V-41 (31) Sta. "LW" 95+811.13 To Sta. "LW" 95+905.69, Rt. Const. Single Slope Conc. Barrier Mod. - 94.6 m Connect To Mod. Median Barrier On Bridge Deck (For Details, See Shts. 2B-4, 2B-5, 2B-7, 2B-12 & Bridge Drgs.) (32) Sta. "LW" 95+967.74 To Sta. "LW" 96+200.44, Rt. Const. Single Slope Conc. Barrier (Reflectorized) - 232.8 m Flare Rate=0.W=0.E=0 Connect To Extg. Conc. Barrier (For Details, See Shts. 2B-4 & 2B-5) OREGON DEPARTMENT OF TRANSPORTATION TERED PROFF ROADWAY ENGINEERING SECTION NGINEER US26: CORNELL RD. -OR217 (BEAVERTON) SEC. 17,856 SUNSET HIGHWAY WASHINGTON COUNTY Design Team Leader - David Joe Polly OPF Designed By - Danny Y. Hori Drafted By - Tien Nguyen SHEET NO. GENERAL CONSTRUCTION NOTES 18A Jun.

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 Sta. "BR" 96+092.55, Lt. Remove Manhole Const. Manhole, Large, 1500 mm Dia. Const. Type "G-2MA" Inlet Inst. 300 mm Storm Sew. Pipe - 19.0 m 3 m Depth Inst. 750 mm Storm Sew. Pipe - 49.5 m 6 m Depth Const. Paved End Slope - 4.6 m²

 Sta. "SC" 96+049.10, Rt. Const. Manhole, Large, 1500 mm Dia. Const. Type "G-1" Inlet - 4 Inst. 300 mm Storm Sew. Pipe - 136 m 3 m Depth Inst. 750 mm Storm Sew. Pipe - 69.0 m 6 m Depth Const. Paved End Slope - 4 9.6 m² (Total)

 Sta. "BR" 96+030.25. Rt. Const. Manhole Const. Type "G-2" Open Graded HMAC Inlet - 2 Inst. 300 mm Storm Sew. Pipe - 11.0 m 1.5 m Depth Inst. 300 mm Storm Sew. Pipe - 64.5 m 3 m Depth Const. Wearing Surface Drain - 32.0 m Const. Wearing Surface Drain Outlet - 2 (See Drg. No. RD314)

 Sta. "LW" 96+041.03.Lt. Const. Manhole, Type Diversion, "High-Low" Inst. 300 mm Storm Sew. Pipe - 34.0 m 1.5 m Depth Const. Paved End Slope - 2.4 m² (For Details, See Sht. GHJ-19)

 (5) Sta. "LW" 96+039.85, Lt. Const. Manhole, Type Pollution Control Const. Type "G-2" Open Graded HMAC Inlet - 2 Inst. 300 mm Storm Sew. Pipe - 53.5 m 1.5 m Depth (For Details, See Sht. GHJ-30)

Sta. "CBR" 95+936.03, Lt. Remove Inlet Adjust Inlet - 2 Const. Type "G-2" Inlet - 2 Inst. 300 mm Storm Sew. Pipe - 6.5 m 1.5 m Depth Inst. 300 mm Storm Sew. Pipe - 9.0 m 3 m Depth

(7) Remove Inlet - 6

(8) Sta. "LE" 96+056.07. Rt. Remove Inlet Const. Type "G-2" Open Graded HMAC Inlet Remove Extg. Pipe - 1.5 m

- (9) Sta. "LE" 95+783.23, Rt. Const. Type "G-2" Open Graded HMAC Inlet - 2 Inst. 300 mm Storm Sew. Pipe - 100.0 m 1.5 m Depth
- (10) Sta. "LW" 95+801.64, Lt. Const. Type "G-2" Inlet Const. Type "G-2" Open Graded HMAC Inlet Inst. 300 mm Storm Sew. Pipe - 111.5 m 3 m Depth Rock Exc. - 12 m³
- Sta. "LW" 95+897.15, Lt. Const. Type "G-2" Inlet Const. Type "G-2" Open Graded HMAC Inlet Inst. 300 mm Storm Sew. Pipe - 11.0 m 3 m Depth

Sta. "D" 95+946.09,Rt. Const. Type "G-2" Open Graded HMAC Inlet Const. Type "G-2MA" Inlet Inlet F.L.92.800 Inst. 300 mm Storm Sew. Pipe - 66.0 m 1.5 m Depth Inst. 3 Piece Elbow - 2 Inst. Slip Joint Inst. Slope Anchor - 6 Connect To Extg. Const. Wearing Surface Drain - 16.5 m Const. Wearing Surface Drain Outlet (For Details, See Sht. GHJ-4)

- (13) Sta. "D" 96+042.07, Rt. Const. Manhole, Type Diversion. "High-Low" Manhole Flow Line, See Sht. GHJ-18 (For Details, See Sht. GHJ-18)
- Sta. "D" 96+028.13, Rt. Const. Manhole, Type Pollution Control Inst. 300 mm Storm Sew. Pipe - 33.5 m 6 m Depth Manhole Flow Line 93.968 Outfall Flow Line 93.600 Const. Paved End Slope - 2.4 m² (For Details, See Sht. GHJ-29)
- (15) Const. Water-Quality Swale "S1" (For Details, See Sht. GHJ-47)
- (b) Sta. "D" 95+984.89, Rt. Const. Manhole, Mod. Inst. 300 mm Storm Sew. Pipe - 37.5 m 1.5 m Depth Manhole Flow Line Match Extg. (93.194±) Outfall Flow Line 90.950 Const. Paved End Slope - 2.4 m² (For Details, See Sht. GHJ-6)

- (17) Sta. "BR" 96+250.48.Lt. Remove Manhole Reconst. Manhole Const. Manhole, Large, 1800 mm Dia. Remove Inlet Const. Type "M-E" Inlet Const. Type "D" Detention Modified Inlet Const. Siphon Box Reconst. Inlet Remove Pipe - 15 m Inst. 450 mm Storm Sew. Pipe - 15.5 m 3 m Depth Inst. 525 mm Storm Sew. Pipe - 34.0 m 3 m Depth Connect To Extg. Const. Loose Riprap Blanket (Class 50) ~ 60 MG 8 m Wide x 8 m Long x 0.45 m Thick Around Inlets Riprop Geotextile, Type 2 - 80 m² Tr. Resurf. - 18 m² (For Details, See Shts. GHJ-5 & GHJ-6)
- (18) Regrade Detention Basin Clearing And Grubbing - 0.8 ha Gen. Exc. - 4500 m³ (For Details, See Sht. GHJ-44)
- (19) Sta. "BR" 96+115.61, L1.
 Const. Loose Riprap Channel (Class 25) 240 MG Riprap Geotextile, Type 2 - 270 m² (For Details, See Sht. GHJ-8)
- Sta. "BR" 96+110.69, Lt. Const. Loose Riprap Basin (Class 100) - 44 MG Riprap Geotextile, Type 2 - 60 m² (For Details, See Sht. GHJ-9)
- Sta. "BR" 96+103.82, Lt. Const. Loose Riprap Channel (Class 350) - 26 MG Riprap Geotextile, Type 2 - 25 m² (For Details, See Sht. GHJ-8)
- (22) Sta. "SC" 96+034.54, Rt. Const. Loose Riprap Channel (Class 25) - 22 MG Riprap Geotextile, Type 2 - 34 m² Dt. Exc. - 14 m³ (For Details, See Sht. GHJ-8)
- (23) Sta. "BR" 96+068.63, Rt. Const. Loose Riprap Channel (Class 25) - 130 MG Riprap Geotextile, Type 2 - 204 m² Dt. Exc. - 81 m³ (For Details, See Sht. GHJ-8)

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- Sta. "BR" 96+067.66, Rt. Const. Loose Riprap Basin (Class 25) - 2.5 MG Riprap Geotextile, Type 2 - 6.5 m² (For Details, See Sht. GHJ-9)
- (25) Const. Access Road
 Aggregate Base (75mm 0) 110 MG
 Subgrade Geotextile 191 m²
 (For Details, See Sht. GHJ-7)
- Sta. "ER" 95+804.38, Rt. Remove Manhole Const. Manhole, Large, 1500 mm Dia. Remove Inlet Remove Pipe - 104.0 m Inst. 750 mm Storm Sew. Pipe - 104.0 m 6 m Depth
- Sta. "ER" 95+908.02, Rt. Const. Manhole, Large, 1500 mm Dia. Const. Type "G-2" Inlet Remove Pipe - 32.5 m Inst. 300 mm Storm Sew. Pipe - 6.0 m 3 m Depth Inst. 750 mm Storm Sew. Pipe - 32.5 m 6 m Depth Tr. Resurf. - 32 m²
- (28) Sta. "ER" 95+940.44. Rt. Remove Manhole
 Const. Manhole. Large, 2400 mm Dia. Connect To Extg. - 6
- Sta. "D" 96+036.93. Rt. Const. Manhole Inst. 300 mm Storm Sew. Pipe - 9.5 m. 3 m Depth Manhole Flow Line 94.100 W., 95.542 E.
 30
- Sta. "CBR" 95+773.05 To Sta. "CBR" 95+857.74, Lt. Const. Conc. Barrier Drain - 83.0 m Drain To Gutter Emb. In Place - 40 m³ (For Details, See Sht. GHJ-1)



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37V-41 1) Const. Detention Basin Removal Of Pvmt. – 1880 m² Clearing And Grubbing – 0.2 ha Gen. Exc. – 3600 m³ (For Details, See Sht.GHJ-46) 2 Remove Inlet - 3 Remove Pipe - 32 m 3 Sta. "D" 95+933.98. Rt. Const. Type "M-E" Detention Modified Inlet Const. Type "D" Inlet Lip Elev. 91.550 Inst. 300 mm Storm Sew. Pipe - 9 m 1.5 m Depth Connect To Existing Trench Resurfacing – 2 m² (For Details, See Sht. GHJ–5) (4) Sta. "D" 95+963.17, Rt. Const. Loose Riprap Channel (Class 25) – 162 MG Riprap Geotextile, Type 2 - 165 m² (For Details, See Sht. GHJ-8) (5) Sta. "D" 95+964.14. Rt. Const. Loose Riprap Basin (Class 25) - 18 MG Riprap Geotextile, Type 2 - 16 m² (For Details, See Sht. GHJ-9) 6 Sta. "D" 95+973.85. Rt. Const. Loose Riprap Channel (Class 25) – 41 MG Riprap Geotextile, Type 2 – 58 m² (For Details, See Sht. GHJ-8) 7 See Sht. 18B-2. Note 16 OREGON DEPARTMENT OF TRANSPORTATION ROADWAY ENGINEERING SECTION US26: CORNELL RD. -Or217 (Beaverton) Sec. SUNSET HIGHWAY WASHINGTON COUNTY eng II Design Team Leader - Eileen J. Phelan Designed By - Henry M. Allen Drafted By - Tien Nguyen SHEET NO. MINTON DRAINAGE & UTILITIES es Dec. 31. 18D



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Extend Plate -Sides To

Outflow

Orifice

Width, (W) Height, (H)

190

190

-







		1									
W1. (m)	D1.	T1.	F.L.U-S	F.L.D-S	Side	slope	Riprop Class	Outlet Structure			
					Left	Right	0,000				
0.6	450	300	66.192	64.839	1:2 1:2		25	Extg. Inlet			
0.6	300	300	68.864	67. 4 00	1:3	1:3	25	Flow Spreader			
0.6	450	450	70.875	69.800	1:2	1:2	25	Wetlands			
0.6	450	450	68.814	68.364	1:2	1:2	25	Flow Spreader			
0.6	300	450	70.073	C3.600	1:4	1:4	25	Tributary 3 Wetlands			
0.6	300	450	70.073	69.600	1:4	1:4	25	Tributary 3 Wetlands			
0.9	300	450	72.670	72.210	1:2	1:2	25	Swale "MA1"			
1.2	600	450	65.700	65.300	1:2	1:2	25	Extg. Ground			
1.2	450	450	66.49 0	66.400	1:2	1:2	25	Extg. Ground			
1.2	600	525	65 .4 60	65.447	1:2	1:2	50	Riprap Channel			
1.8	600	525	71.350	59.600	1:2	1:2	50	Johnson Creek			
1.2	450	4 50	69.800	60.200	1:2	1:2	25	Johnson Creek			
2.4	150	450	82.00 5	81.760	1:4 1:4		25	Downstream Swale			
2.4	150	450	81.500	80. 9 60	1:4	1:4	25	Downstream Swale			
2.4	150	450	80.700	80.400	1:4	1:4	25	Downstream Swale			
2.4	150	450	7 9.9 00	79.800	1:4	1:4	25	Downstream Swale			
1.2	450	450	90.950	90.800	1:2	1:2	25	ME Inlet			
2.4	300	450	93.370	90.950	1:2	1:2	25	Riprap Basin			
).6	300	450	101.800	101.280	1:4	1:4	25	Riprap Channel			
).6	300	4 50	101.650	101.200	1:4	1:4	25	Inlet			
.6	300	450	104.241	104.200	1:4	1:4	25	Flow Spreader			
.5	450	750	94.000	93.300	1:2	i:2	350	Riprap Basin			
.5	600	450	93.300	93.100	1:2	1:2	25	ME Inlet			

RIPRAP BASIN





PLAN

3

VIEW 3



SECTION B - B

Location Center Of Basin	L2. (m)	W2, (m)	D2. (mm)	T2. (mm)	Design Elevation (m)	Riprap Class
Sta. "LE"95+107.49, Rt.	5	5	450	4 50	5 9 .750	25
"LE"95+268.02, Rt	5.8	3.8	750	600	71.800	100
"LW"95+309.62, Lt.	3.6	3.6	600	600	70.100	50
"D"95+964.14, Rt	2.4	2.4	300	450	90.950	25
"SC"96+067.55, Rt	1	2.4	0	450	101.935	25
"BR"96+067.66, Rt	1	2.4	0	450	101.650	25
"BR"96+110.69, Rt.	4. 8	4.8	1200	600	9 3.9 00	100

37V-41 All Dimensions Are In Millimeters (mm) Unless Otherwise Noted. OREGON DEPARTMENT OF TRANSPORTATION GEO / HYDRO SECTION US26: CORNELL RD. -OR217 (BEAVERTON) SEC. SUNSET HIGHWAY WASHINGTON COUNTY Project Leader - Naveen Chandra Designed By - Henry M. Allen Drafted By - Mortin G. Cosillas SHEET MINTON 200 WATER QUALITY DETAILS es Dec. 31. GHJ-9











Notes: 1) U-S= Upstream, D-S= Downstream 2) See Site Plans For Pipe Inverts At Inlets. 3) "C-T Blend" = Compost-Topsoil Blend, "Rock+C-T" = Drain Rock With Compost-Topsoil Blend.

						Centerline									
					Long.	Curve					Freeboard	Swale	No. Under		
	L,	W,	F.L. U-S,	F.L. D-S,	Slope,	Radius,		Swale Side	eslopes		Depth,	Bottom	Drain	Under Drain Tie-In	Swale Outlet
Swale ID	m	m	m	m	%	m	U-S	Left	Right	D-S	m	Medium	Segments	Location	Facility
WCW	340	1.2	See GHJ-32	See GHJ-32	Varies	None	1:3	1:4	1:6	1:4	0.3	Rock+C-T	2	"G-2MA" Mod. Inlet	"G-2MA" Mod. Inlet
WCE	322	2.4	See GHJ-33	See GHJ-33	Varies	None	1:20	1:6	1:4	1:18	0.3	Rock+C-T	2	"G-2MA" Mod. Inlet	"G-2MA" Mod. Inlet
WC1	82	2.4	68.062	66.543	1.85	None	1:4	1:3	1:6	1:4	0.3	C-T Blend	2	"D" Mod. Inlet	"M-E" Mod. Inlet
WC2A	30	0.6	70.673	70.197	1.50	None	1:2	1:2	1:2	1:2	0.3	C-T Blend	1	"D" Mod. Inlet	"D" Mod. Inlet
WC2B	474	0.7	70.815	70.637	0.32	None	1:3	1:3	1:3	1:3	0.3	C-T Blend	1	"D" Mod. Inlet	"D" Mod. Inlet
WC2C	37	0.8	71.042	70.839	0.55	None	1:4	1:4	1:4	1:4	0.45	C-T Blend	1	"D" Mod. Inlet	"D" Mod. Inlet
WC2D	41	0.9	72.556	71.634	Varies	None	1:5	1:5	1:5	1:5	0.3	Rock+C-T	1	"D" Mod. Inlet	"D" Mod. Inlet
WC3A	50	2.4	70.195	69.961	0.5	None	1:4	Var.	Var.	1:4	.8	C-T Blend	2	"D" Mod. Inlet	"V"-Bottom Ditch
WC3B	50	2.4	74.408	74.158	0.5	80	1:4	1:4	1:6	1:4	0.45	C-T Blend	2	"D" Mod. Inlet	"M-E" Mod. Inlet
MA1	31.5	2.4	72.160	72.000	0.51	None	1:3	1:3	Var.	1:6	0.45	C-T Blend	None	N.A.	"M-E" Mod. Inlet
CBR	See GHJ-43	2.4	See GHJ-43	See GHJ-43	Varies	None	1:4	1:4	1:4	1:4	0.45	C-T Blend	None	N.A.	"D" Inlet
N1a	23	2.4	102.150	102.035	0.5	100	1:4	1:4	1:4	1:4	0.45	C-T Blend	None	N.A.	Riprap Basin
N1b	12.3	2.4	101.812	101.750	0.5	25	1:4	1:4	1:4	1:4	0.45	C-T Blend	None	N.A.	Channel
N2	36	2.4	102.750	102.570	0.5	100	1:4	1:4	1:4	1:4	0.45	C-T Blend	None	N.A.	Pipe
N3	36	2.4	103.350	103.170	0.5	100	1:4	1:4	1:4	1:4	0.45	C-T Blend	None	N.A.	Pipe
N4	36	2.4	103.850	103.670	0.5	100	1:4	1:4	1:4	1:4	0.45	C-T Blend	None	N.A.	Pipe
N5a	12.6	2.4	104.404	104.341	0.5	None	1:4	1:4	1:4	1:4	0.45	C-T Blend	None	N.A.	Channel
N5b	24	2.4	104.150	104.030	0.5	100	1:4	1:4	1:4	1:4	0.45	C-T Blend	None	N.A.	Pipe
S1	36	2.4	93.550	93.370	0.5	20, Each	1:4	1:4	1:4	1:4	0.45	C-T Blend	None	N.A.	Channel



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