OPERATION & MAINTENANCE MANUAL

DFI No.: D00036

Facility Type: Detention Tank/Pipe



MARCH, 2011

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1. Identification

Drainage Facility ID (DFI): **D00036**

Facility Type: Detention Tank/Pipe

Construction Drawings: (V-File Number) 38V-117

Location: District: 3

Highway No.: 001

Mile Post: 252.16 / 252.19 (beg./end)

Description: This facility is located along the

centerline of I-5 (Hwy 001) in the left shoulder/median of the northbound lanes, between two bridge structures, overcrossing Turner Road (Bridge No. 20032) and the U.P.R.R. (Bridge No. 20026) rail line.

2. Facility Contact Information

Contact the Engineer of Record, Region Technical Center, or Geo-Environmental's Senior Hydraulics Engineer for:

- Operational clarification
- Maintenance clarification
- Repair or restoration assistance

Engineering Contacts:

Region Technical Center Hydro Unit Manager

Or

Geo-Environmental Senior Hydraulics Engineer (503) 986-3365.

3. Construction

Engineer of Record:

ODOT Designer – Region 2 Tech. Center

Chris Carman, 503-986-2691

Facility construction: 2005

Contractor: Hamilton Construction Company

4. Storm Drain System and Facility Overview

A detention facility is designed to control the quantity of runoff, by reducing the peak discharge and only detaining runoff for some short period of time. These facilities are designed to store and gradually release or attenuate stormwater runoff via a control structure or release mechanism, and completely drain after the design storm has passed. The most common detention facilities include:

- Dry ponds these are depressed storage areas that store runoff during wet weather and are dry the rest of the time. Usually they are earthen depressions.
- Tanks these are underground storage facilities that are typically constructed from large diameter pipe.
- Vaults these are enclosed underground storage facilities. They are typically constructed from reinforced concrete.

The detention pipe facility is located along the centerline of I-5 (Hwy 001) in the left shoulder/median of the northbound lanes, between two bridge structures, overcrossing Turner Road (Bridge No. 20032) and the U.P.R.R. (Bridge No. 20026) rail line.. The facility can be accessed from the northbound travel lanes (Photo 1).

The detention pipe facility is composed of a 72-inch diameter pipe segment, approximately 105 feet long, and two oversized manholes. The manholes include an upstream manhole and a flow control manhole (Point A on Operational Plan in Appendix A). This facility receives flow from both the north and south ends. From the west, a 12-inch diameter, 80-foot long pipe conveys stormwater from facility DFI D00037 to the flow control manhole of this facility, DFI From the north, a 12-inch diameter, 40-foot long pipe D00036. conveys stormwater to the flow control manhole from a manhole tied with two inlets (Photo 2). From the south, stormwater runoff is collected by one inlet and discharged into the south manhole (Photo 3). A 15-inch diameter, 60-foot long pipe conveys stormwater from the flow control manhole eastwards to a downstream facility, DFI D00033 (Operational Plan in Appendix A; 16C & 16D in 38V-117 in Appendix B).

(Note: The southern and northern manholes of D00036 and the water quality structure for D00033 were not found during fieldwork. Construction photographs confirm that the northern manhole of D00036 exists. The southern manhole of D00036 and the water quality structure for D00033 may exist.)

The roadway within this area slopes downward towards the north. The drainage area for this detention facility includes the area from

the southern end panel of the U.P.R.R. overcrossing to the southern end panel of the Turner Road overcrossing (Operational Plan in Appendix A; 16C & 16D in 38V-117 in Appendix B; Photos 4 & 5). Stormwater runoff from the northbound lanes of I-5 (Hwy 0001) enters the detention system through inlets either north or south of the flow control manhole (Photos 2 & 3). Conveyance pipes deliver the stormwater runoff first to manholes and then to the flow control manhole. Stormwater runoff from the southbound lanes of I-5 (Hwy 001) passes through the inlets, conveyance pipes and manholes of D00037 before being discharged into the flow control manhole of D00036 from a 12-inch diameter, 80-foot long conveyance pipe (Operational Plan in Appendix A; 16C & 16D in 38V-117 in Appendix B).

After detention in the flow control manhole, the water is released into a 15-inch diameter storm pipe that discharges into a water quality structure (DFI D00033) approximately 60 feet east. DFI D00033 discharges treated stormwater into a 15-inch diameter, 77-foot long storm pipe, which conveys the water downhill and farther east to an inlet structure. From the inlet structure, an 18-inch diameter, 184-foot long storm pipe conveys the water to the facility's ultimate outfall at a ditch north of the U.P.R.R. railroad, which runs under Bridge No. 20026 (Operational Plan in Appendix A; 16C & 16D in 38V-117 in Appendix B; Photo 6).

For further information and details regarding the system refer to **Appendix A** for the **Operational Plan** and **Appendix B** for **Construction Drawings**.

A. Maintenance equipment access:

This facility is located along the centerline in the median/12 foot-wide left shoulder of the northbound lanes of I-5 (Hwy 001). Sufficient room to utilize the shoulder area should allow adequate access to the system when performing maintenance activities such as using a vactor truck to clean the manholes.

B.	. Heavy equipment access into facility:		
	☑ Allowed (no limitations)☐ Allowed (with limitations)☐ Not allowed		
C.	Special Features:		
	☐ Amended Soils ☐ Porous Pavers		

☐ Liners☐ Underdrains

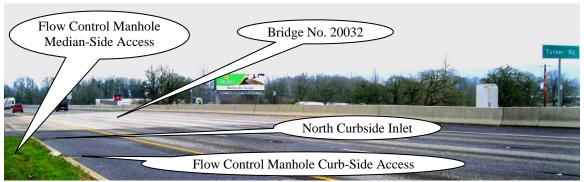


Photo 1: D00036 across from Turner Road sign.



Photo 2: Northern inlets. The manhole between these inlets was not found during fieldwork, but construction photographs confirm that the manhole exists. For the position of this manhole, refer to the Operational Plan in Appendix A and pages 16C and 16D in 38V-117 in Appendix B.



Photo 3: Southern inlet. The manhole attached to this inlet was not found during fieldwork, but it may exist. For the position of this manhole, refer to the Operational Plan in Appendix A and pages 16C and 16D in 38V-117 in Appendix B.



Photo 4: Drainage area. Photograph taken facing north.

- 5 **-**

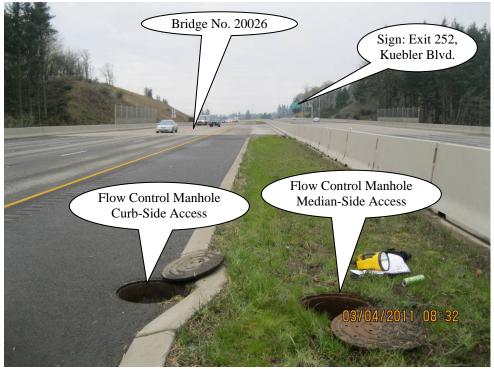


Photo 5: Drainage area. Photograph taken facing south.



Photo 6: Outfall pipe to ditch. Photograph taken facing east. (U.P.R.R. to right (south) not pictured.)

- 6 -

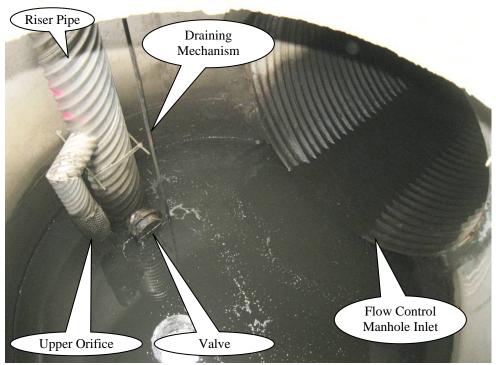


Photo 7: Flow control manhole interior view from median-side access.

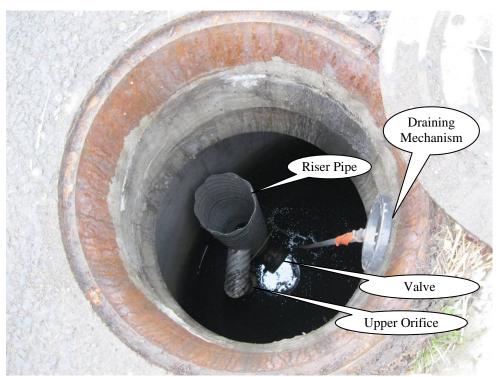


Photo 8: Flow control manhole interior view from curb-side access.

- 7 -

5. Facility Haz Mat Spill Feature(s)

This detention facility does not have features to block liquids from draining from the pipe. However, the detention pipe itself can be used to store hazardous liquids entering the system until such time the pipe is full and begins flowing toward the water quality structure (to the east) where additional storage of these liquids may be available in the manhole's sump. Another option may be possible by blocking the outfall pipe downstream from this water quality structure and capturing hazardous liquids there.

6. Auxiliary Outlet

Auxiliary Outlets are provided if the primary outlet control structure can not safely pass the projected high flows. Broad-crested spillway weirs and over flow risers are the two most common auxiliary outlets used in stormwater treatment facility design. The auxiliary outlet feature is either a part of the facility or an additional storm drain feature/structure.

The auxiliary outlet feature for this facility is:

□ Designed into facility:

In the event that the orifices become plugged or the flows exceed the capacity of the facility, the water is released through the high flow riser within the flow control structure.

The auxiliary high flow bypass for the flow control manhole consists of a riser pipe above the outlet flow pipe in the flow control manhole. If stormwater enters the flow control manhole more quickly than the lower orifice can convey stormwater (due to either a larger storm or clogging of the lower orifice), the water level within the manhole will rise until water enters the riser pipe through its upper orifice. The water will then discharge through the high flow riser. (Flow Control Structure Detail in the Operational Plan in Appendix A; working schematics on sheets GJ-4 & GJ-6 in 38V-117 in Appendix B; Photos 7 & 8).

If the lower orifice clogs, and if the flow control manhole fills with water, use of the draining mechanism will quickly reduce the water level inside the manhole. The draining mechanism opens a valve in the riser pipe, thus enabling flows to leave directly through the flow control manhole outlet and bypass the lower orifice, the upper orifice and the top of the riser pipe (**Operational Plan in Appendix A**; **Photos 7 & 8**).

7. Maintenance Requirements

Routine maintenance table for non-proprietary stormwater treatment and storage/detention facilities have been incorporated into ODOT's Maintenance Guide. These tables summarize the maintenance requirements for ponds, swales, filter strips, bioslopes, and detention tanks and vaults. Special maintenance requirements in addition to the routine requirements are noted below when applicable.

The ODOT Maintenance Guide can be viewed at the following website:

http://www.oregon.gov/ODOT/HWY/OOM/MGuide.shtml

Maintenance requirements for proprietary structures, such as underground water quality manholes and/or vaults with filter media are noted in Appendix C when applicable.

The following stormwater facility maintenance table (See ODOT Maintenance Guide) should be used to maintain the facility outlined in this Operation and Maintenance Manual or follow the Maintenance requirements outlined in Appendix C when proprietary structure is selected below:

☐ Table 2 (stormwater ponds)
☐ Table 3 (water quality biofiltration swales)
☐ Table 4 (water quality filter strips)
☐ Table 5 (water quality bioslopes)
∑ Table 6 (detention tank)
☐ Table 7 (detention vault)
☐ Appendix C (proprietary structure)
☐ Special Maintenance requirements:

Note: Special maintenance Requirements Require Concurrence from ODOT SR Hydraulics Engineer.

8. Waste Material Handling

Material removed from the facility is defined as waste by DEQ. Refer to the roadwaste section of the ODOT Maintenance Yard Environmental Management System (EMS) Policy and Procedures Manual for disposal options:

http://egov.oregon.gov/ODOT/HWY/OOM/EMS.shtml

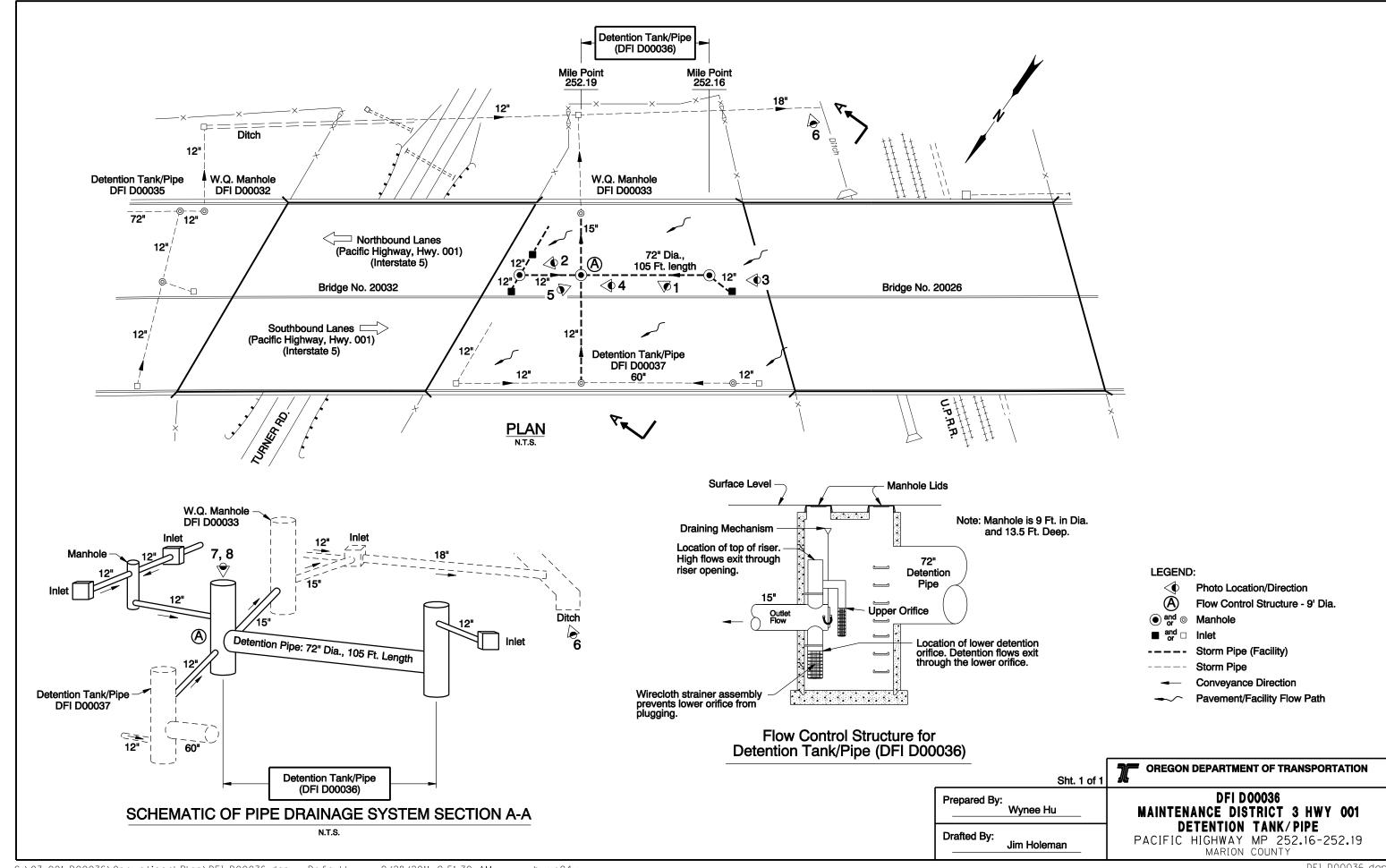
Contact any of the following for more detailed information about management of waste materials found on site:

ODOT Clean Water Unit	(503) 986-3008
ODOT Statewide Hazmat Coordinator	(503) 229-5129
ODOT Region Hazmat Coordinator	(503) 986-2647
ODEQ Northwest Region Office	(503) 229-5263

Appendix A

Content:

Operational Plan and Profile Drawing(s)



Appendix B

Content:

- ODOT Project Plan Sheets
 - o Cover/Title Sheet
 - o Water Quality/Detention Plan Sheets
 - o Other Details

INDEX OF SHEETS		
SHEET NO. DESCRIPTION		
1	Title Sheet	
1A	Index Of Sheets Cont'd.	
1A-2	Index Of Sheets Cont'd.	
1A-3	Index Of Sheets Cont'd.	
1A-4	Standard Drawing Nos.	
1B	Layout Sheet	

STATE OF OREGON DEPARTMENT OF TRANSPORTATION

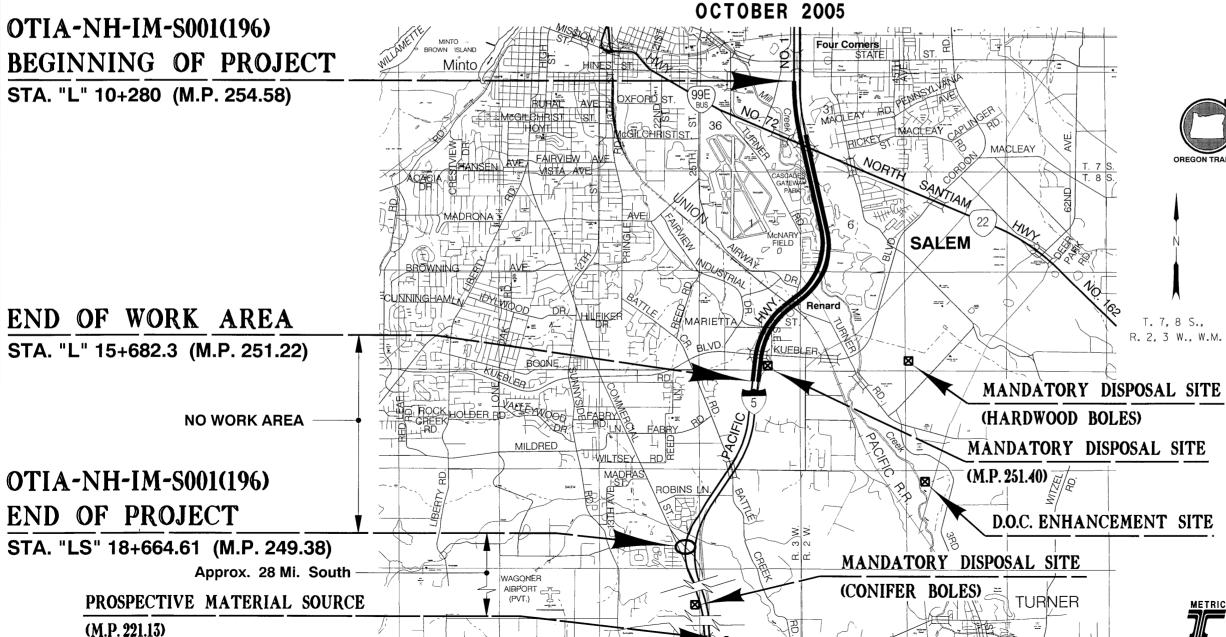
PLANS FOR PROPOSED PROJECT

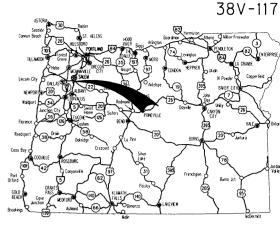
GRADING, DRAINAGE, STRUCTURES, PAVING, SIGNING, ILLUMINATION, SIGNALS & ROADSIDE DEVELOPMENT

I-5: N. SANTIAM HWY. -KUEBLER BLVD. (SALEM) SEC.

PACIFIC HIGHWAY

MARION COUNTY





Overall Length Of Project - 4.02 km (2.49 Miles)

ATTENTION:

Oregon Law Requires You To Follow Rules
Adopted By The Oregon Utility Notification
Center. Those Rules Are Set Forth In
OAR 952-001-0010 Through OAR 952-001-0090.
You May Obtain Copies Of The Rules By Calling
The Center. (Note: The Telephone Number For
The Oregon Utility Center Is (503) 232-1987.)



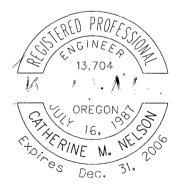


OREGON TRANSPORTATION COMMISSION

Stuart Foster CHAIRMAN
Gail L. Achterman COMMISSIONER
Mike Nelson COMMISSIONER
Randall Papé COMMISSIONER
Janice J. Wilson COMMISSIONER

Bruce A. Warner

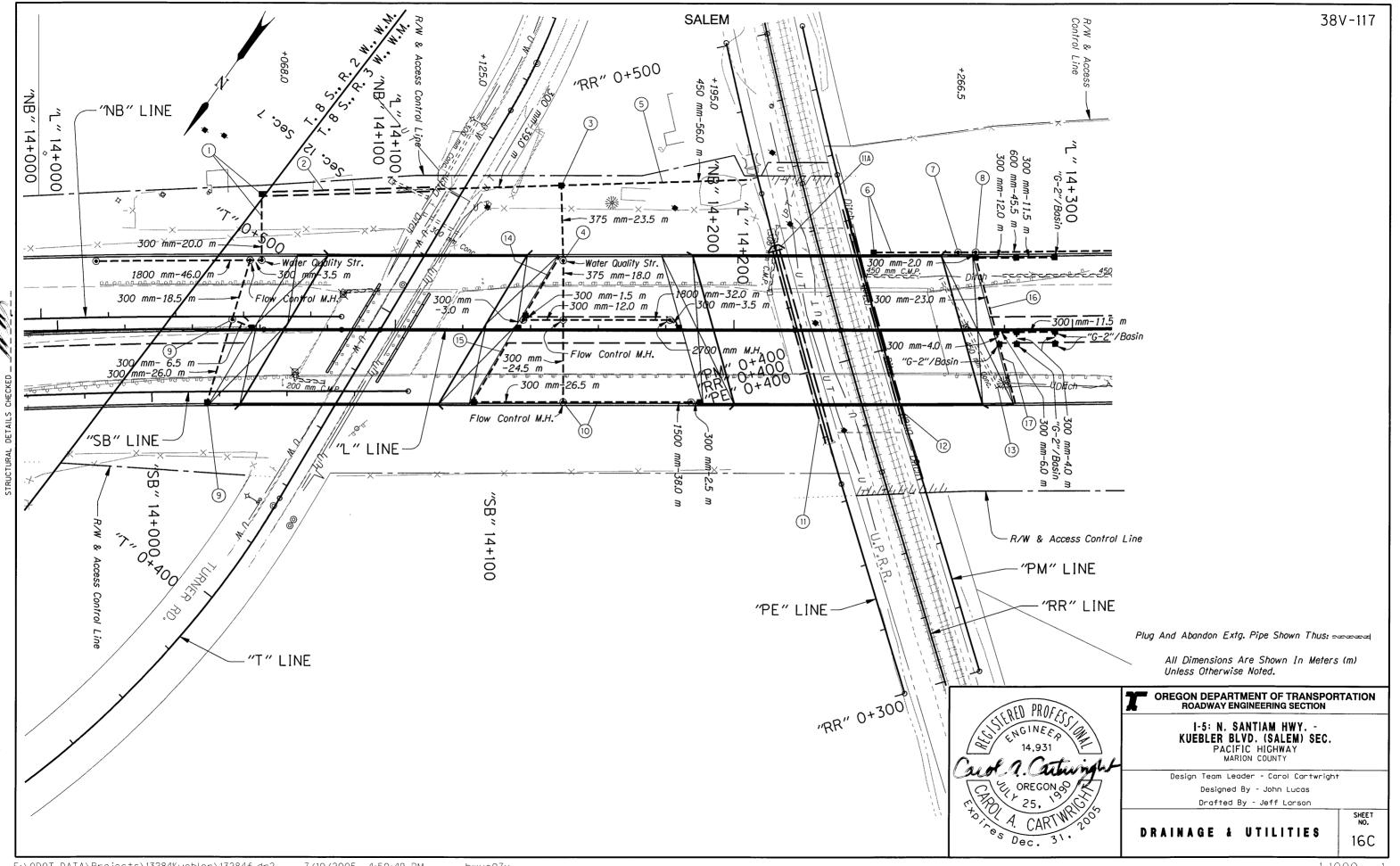
DIRECTOR OF TRANSPORTATION



Catherine M. Nelson
TECHNICAL SERVICES MANAGING ENGINEER

I-5: N. SANTIAM HWY. -KUEBLER BLVD. (SALEM) SEC. PACIFIC HIGHWAY

FEDERAL HIGHWAY ADMINISTRATION	PROJECT NUMBER	SHEET NO.
OREGON DIVISION	OTIA-NH-IM-S001(196)	1

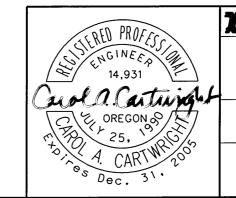


- ① Sta. "L"14+051.1 To Sta. "NB"14+065.4 Const. Manhole 2700 mm Dia. Const. Water Quality Structure Const. Flow Control Manhole 2400 mm Dia. Const. Siphon Box Inst. 300 mm Sew. Pipe - 18.5 m 3.0 m Depth Inst. 300 mm Sew. Pipe - 23.5 m 6.0 m Depth Inst. 1800 mm Sew. Pipe - 46.0 m 6.0 m Depth (For Details, See Sht. GJ-4) (See Drg. No. RD376)
- 2 Const. Ditch "V" Bottom, 1:3 Slopes Dt. Exc. - 48 m³
- (3) Sta. "L"14+110.1 To Sta. "L"14+148.9 Const. Type "G-2MA" Inlet Shape Bottom Inst. 300 mm Sew. Pipe - 39.0 m 1.5 m Depth Inst. 375 mm Sew. Pipe - 23.5 m 6.0 m Depth Inst. Slope Anchors (See Drg. Nos. RD330 & RD364)
- (4) Sta. "L"14+136.4 To Sta. "L"14+183.9 Const. Water Quality Structure Const. Flow Control Manhole 2700 mm Dia. Const. Large Manhole 2700 mm Dia. Const. Manhole Const. Type "G-2" Open Grade HMAC Inlet With Basin - 2 0.45 m Deep Const. Type "G-2" Open Grade HMAC Inlet Shape Bottom Adjust Inlet For Wearing Course - 2 Inst. 300 mm Sew. Pipe - 20.0 m 1.5 m Depth Inst. 300 mm Sew. Pipe - 24.5 m 3.0 m Depth Inst. 375 mm Sew. Pipe - 18.0 m 6.0 m Depth Inst. 1800 mm Sew. Pipe - 32.0 m 6.0 m Depth (For Details, See Sht. GJ-4)
- (5) Sta. "L"14+148.9 To Sta. "L"14+205.0 Inst. 450 mm Sew. Pipe - 56.0 m 1.5 m Depth

hwye07x

- (6) Sta. "L"14+241.4 To Sta. "L"14+266.3 Const. Type "D MOD" Inlet Inst. 300 mm Sew. Pipe - 26.5 m 1.5 m Depth Inst. Slope Anchors (For Details, See Sht. GJ-9) (See Dra. No. RD370)
- (T) Sta. "L" 14+266.3 To Sta. "L" 14+271.5 Const. Water Quality Structure Inst. 600 mm Sew. Pipe - 5.5 m 1.5 m Depth
- (8) Sta. "L"14+271.5 To Sta. "L"14+317.4 Const. Manhole Const. Type "G-2" Open Grade HMAC Inlet With Basin - 5 0.45 m Deep Const. Type "G-2" Open Grade HMAC Inlet - 4 Shape Bottom Adjust Inlet For Wearing Course - 6 Inst. 300 mm Sew. Pipe - 78.0 m 1.5 m Depth Inst. 600 mm Sew. Pipe - 45.5 m 1.5 m Depth
- (9) Sta. "SB"14+016.3 To Sta. "L"14+051.1 Const. Manhole Const. Type "G-2" Open Grade HMAC Inlet With Basin - 2 0.45 m Deep Adjust Inlet For Wearing Course - 2 Inst. 300 mm Sew. Pipe - 6.5 m 1.5 m Depth Inst. 300 mm Sew. Pipe - 26.0 m 3.0 m Depth
- (10) Sta. "L"14+123.4 To Sta. "L"14+190.1 Const. Large Manhole 2100 mm Dia. Const. Flow Control Manhole 2100 mm Dia. Const. Type "G-2" Open Grade HMAC Inlet With Basin - 2 0.45 m Deep Adjust Inlet For Wearing Course - 2 Inst. 300 mm Sew. Pipe - 2.5 m 1.5 m Depth Inst. 300 mm Sew. Pipe - 26.5 m 3.0 m Depth Inst. 1500 mm Sew. Pipe - 38.0 m 6.0 m Depth (For Details, See Sht. GJ-4)

- (1) Remove Extg. 2.1 m x 1.2 m R.C.B.C. Const. Channel Change (IIA) Const. Outlet (For Details, See Shts, GE-1, GE-2, GE-3 & GF-1)
- (12) Const. Channel Change (For Details, See Sht. GF-2)
- (13) Remove Pipe
- (14) Sta. "L"14+135.6 Const. Open Grade Wearing Surface Drain Outlet To Inlet
- (15) Sta. "L"14+135.6 Const. Open Grade Wearing Surface Drain Outlet To Inlet
- (16) Sta. "L"14+272.0 Const. Open Grade Wearing Surface Drain Outlet To Inlet
- (17) Sta. "L"14+278.5 Const. Open Grade Wearing Surface Drain Outlet To Inlet



OREGON DEPARTMENT OF TRANSPORTATION **ROADWAY ENGINEERING SECTION**

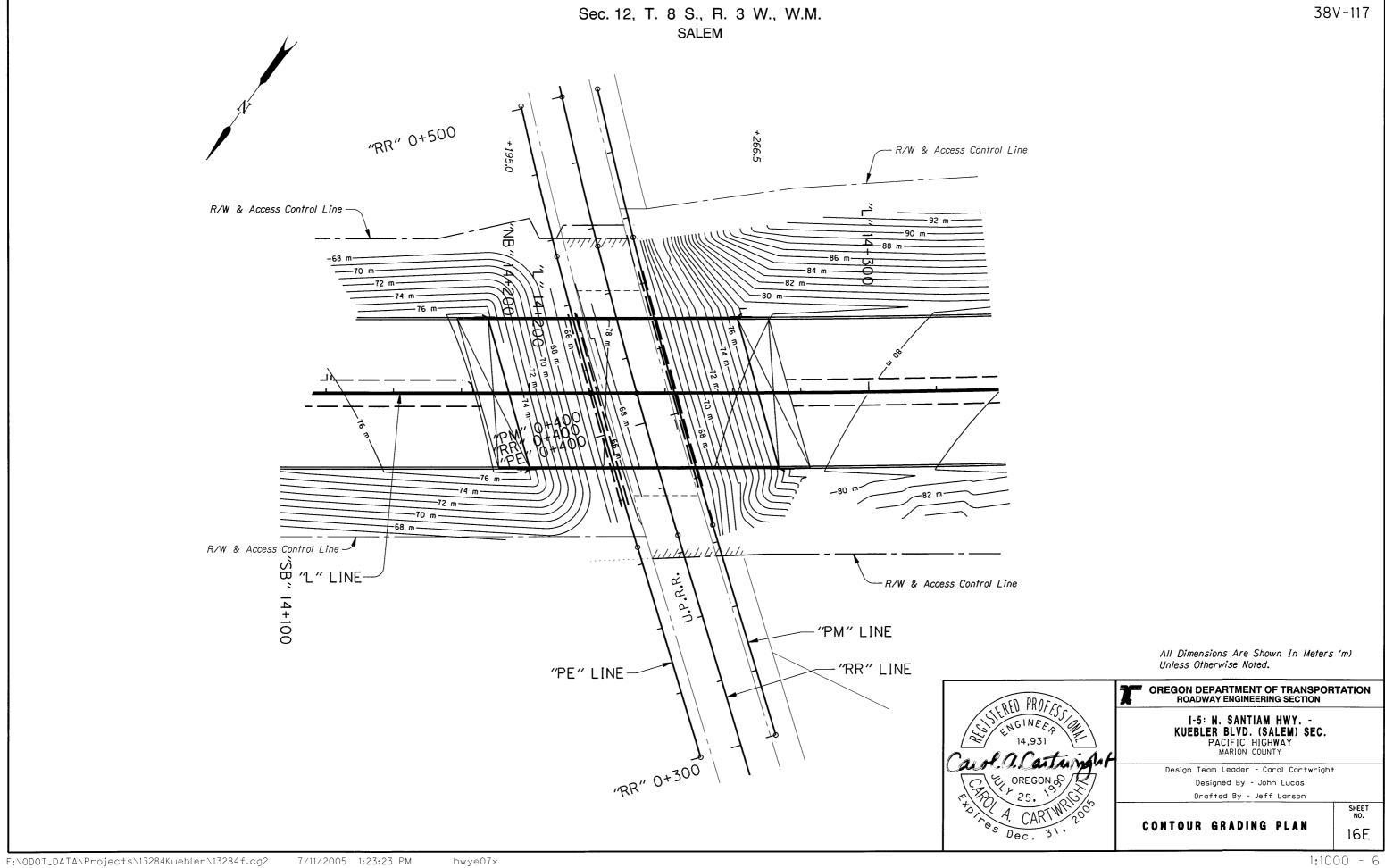
I-5: N. SANTIAM HWY. -KUEBLER BLVD. (SALEM) SEC. PACIFIC HIGHWAY

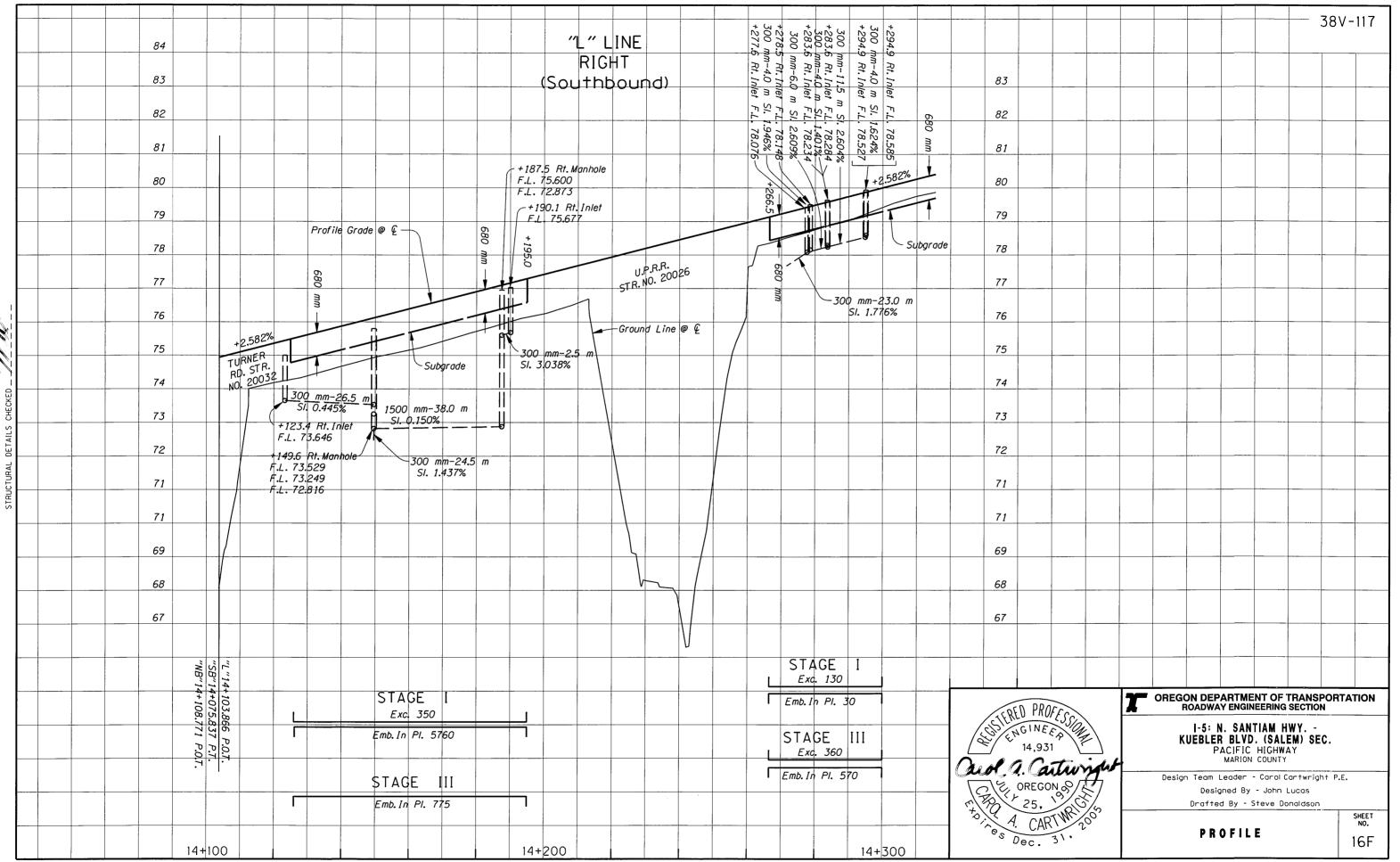
Design Team Leader - Carol Cartwright Designed By - John Lucas Drafted By - Jeff Larson

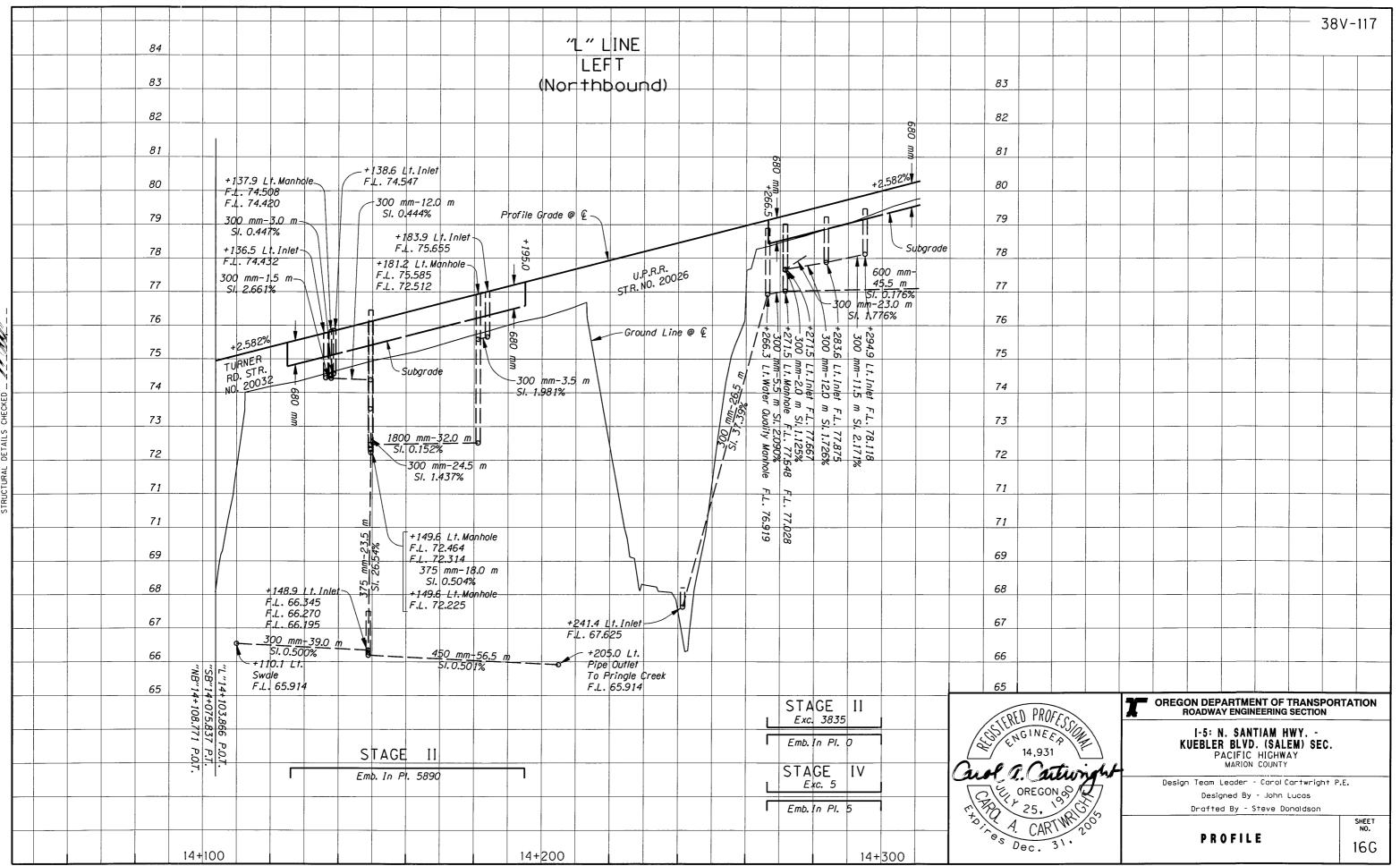
NOTES

SHEET NO. 16D

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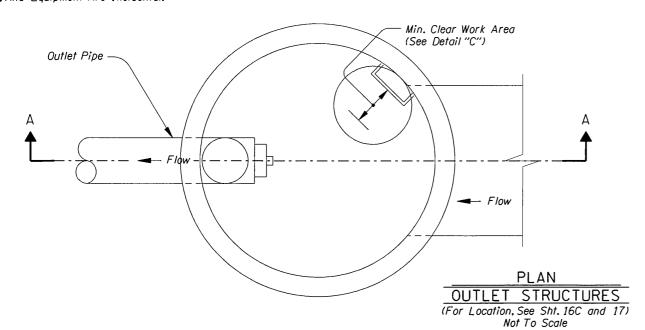


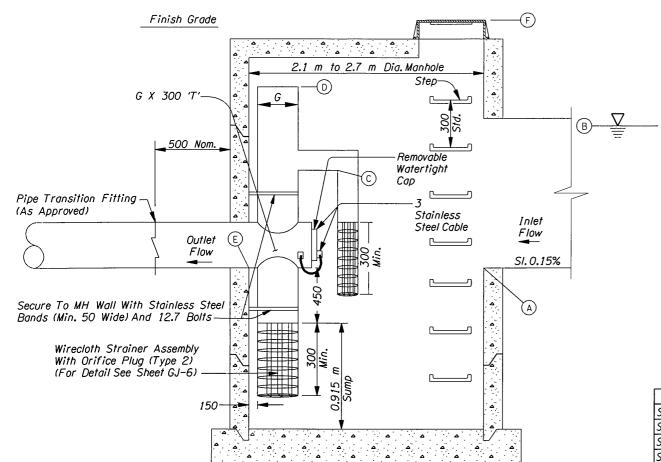




NOTES:

- 1. Hardware, Fasteners And Anchors To Be Stainless Steel; Use 3 mm Stainless Steel Cable.
- 2. For Manhole Details Not Shown, See RD346
- 3. Hardware, Fasteners, Anchors, Fittings, Appurtenances, Labor, And Equipment Are Incidental.

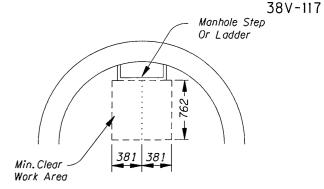




Sta "L" 14+149.633 21.510 Rt.		
	ELEVATION (m)	DESCRIPTION
Α	73.251	Detention Pipe Inlet
В	74.733	Elev. Of Detention Water Surface 50 Year Storm
С	74.247	Fl. Elev. Of Elbow
D	75.033	Rim Of Overflow Riser
E	73.247	Fl. Elev. Of Outlet Pipe
F	75.796	Top Of Manhole

Sto "L" 14+149.601 0.720 Lt.		
	ELEVATION (m)	DESCRIPTION
Α	72.466	Detention Pipe Inlet
В	74.131	Elev. Of Detention Water Surface 50 Year Storm
С	73.058	Fl. Elev. Of Elbow
D	74.431	Rim Of Overflow Riser
Ε	72.458	Fl. Elev. Of Outlet Pipe
F	76.124	Top Of Manhole

	Sta "L" 14+061.972 16.794 Lt.			
	ELEVATION (m)	DESCRIPTION		
Α	69.814	Detention Pipe Inlet		
В	71.408	Elev. Of Detention Water Surface 50 Year Storm		
С	70.353	Fl. Elev. Of Elbow		
D	71.708	Rim Of Overflow Riser		
E	69.803	Fl. Elev. Of Outlet Pipe		
F	74.439	Top Of Manhole		



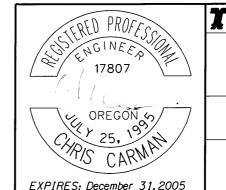
Locate Pipes, Etc. So That No Portion Of Them Are Within Min. Clear Work Area

	DETAI	<u>L_″C″</u>	9
MIN.	CLEAR	WORK	AREA
	Not To	Scale	

Sta "L" 14+362.421 25.372 Lt.			
ELEVATION (m) DESCRIPTION		DESCRIPTION	
Α	77.376	Detention Pipe Inlet	
В	78.839	Elev. Of Detention Water Surface 50 Year Storm	
С	78.071	Fl. Elev. Of Elbow	
D	79.139	Rim Of Overflow Riser	
E	77.371	Fl. Elev. Of Outlet Pipe	
F	80.618	Top Of Manhole	

Sta "L" 14+360.118 2.547 Rt.				
	ELEVATION (m)	TION (m) DESCRIPTION		
Α	78.348	Detention Pipe Inlet		
В	79.723	Elev. Of Detention Water Surface 50 Year Storm		
С	79.043	FI. Elev. Of Elbow		
D	80.023	Rim Of Overflow Riser		
E	78.343	Fl. Elev. Of Outlet Pipe		
F	81.590	Top Of Manhole		

All Dimensions Shown Are In Millimeters (mm) Unless Otherwise Noted



' (REGON DEPARTMENT OF TRANSPORTATION REGION 2 TECH CENTER

I-5: NORTH SANTIAM HWY. -KUEBLER BLVD. (SALEM) SEC. PACIFIC HIGHWAY

MARION COUNTY

Reviewed By - Alvin Shoblom

Designed By - Chris Carman

Designed By - Chris Carman

Drafted By - Chris Shearer

DETAILS

Location G(mm)

Sto. "L" 14+149.633 21.510 Rt. 250

Sta. "L" 14+149.601 0.720 Lt. 250

Sta. "L" 14+061.972 16.494 Lt. 250

Sta. "L" 14+362.421 25.372 Lt. 300

Sto. "L" 14+360.118 2.547 Rt. 450

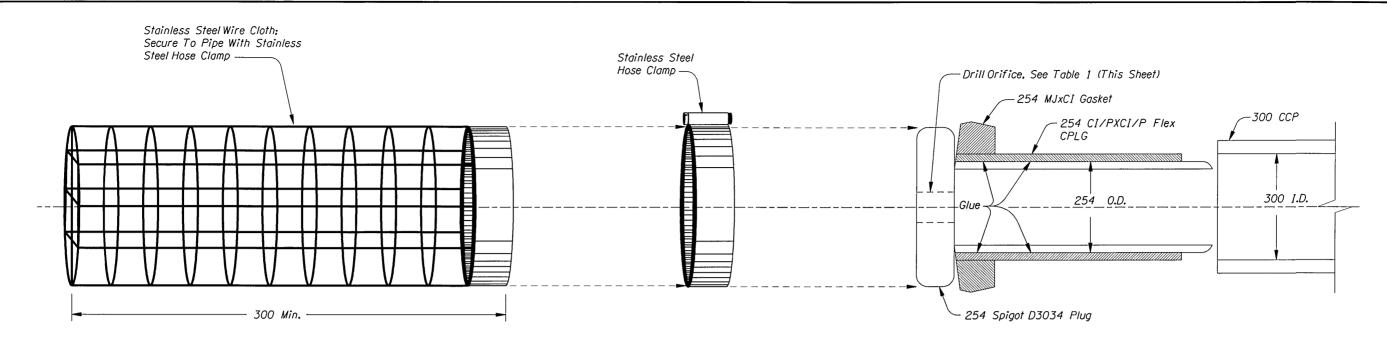
SECTION A-A

FLOW CONTROL MANHOLE

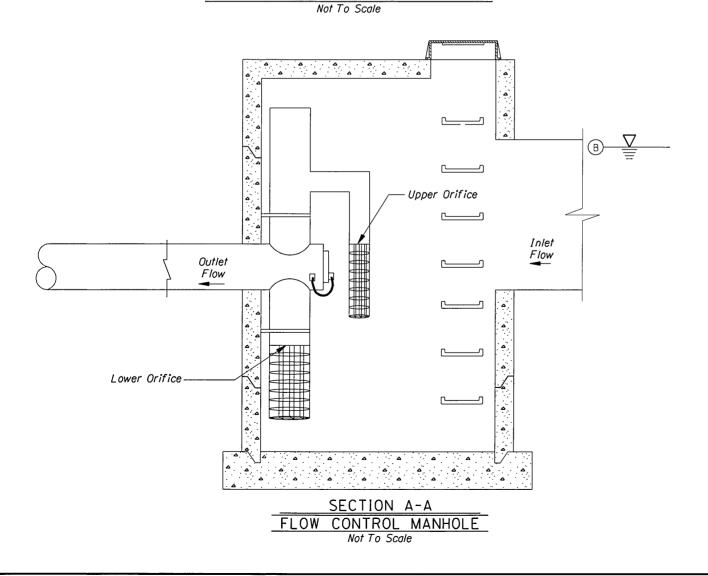
Not To Scale

SHEET NO.

GJ-4



FLOW CONTROL MANHOLE WIRE CLOTH STRAINER ASSEMBLY



ORIFICE PLUG (TYPE 2) Not To Scale

Table 1

Location	Lower Orifice Dia.(mm)	Upper Orifice Dia.(mm)
North Santiam Interchange	63.5	N/A
Sta."L" 14+360.118 2.547 Rt.	50	250
Sta."L" 14+362.421 25.372 Lt.	25	175
Sta. "L" 14+149.633 21.50 Rt.	25	75
Sta."L" 14+149.601 0.720 Lt.	50	115
Sta."L" 14+061.976 16.794 Lt.	25	75
Kuebler Blvd. Interchange	25	400

All Dimensions Shown Are In Millimeters (mm) Unless Otherwise Noted

